

## 가로보 검토

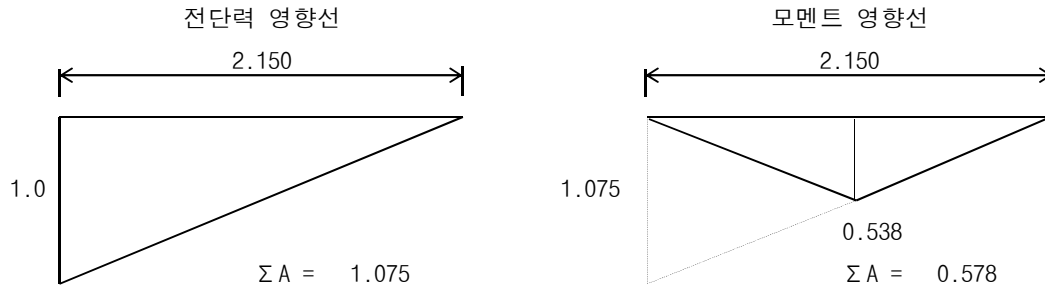
1. 가로보 검토(강도설계법) B =1700mm, T = 450mm (DB-18)

### 1.1 단부 가로보

#### 1.1.1 작용 단면력

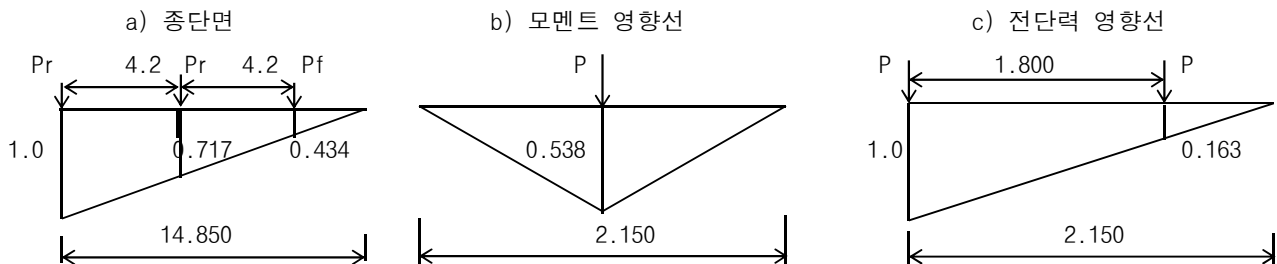
빔간격 = 2.300 m , L = 2.150 m

#### ① 고정하중 (Dead Load)



| 구 분 | 두 개   | 폭     | 단위중량  | 전단력    | 종 거   | 모멘트    |
|-----|-------|-------|-------|--------|-------|--------|
| 포 장 | 0.080 | 2.980 | 2,300 | 5.894  | 0.538 | 3.168  |
| 슬래브 | 0.170 | 2.980 | 2,500 | 13.615 | 0.538 | 7.318  |
| 가로보 | 1.700 | 0.450 | 2,500 | 20.559 | 0.538 | 11.051 |
| 합 계 |       |       |       | 40.069 |       | 21.537 |

#### ② 활 하 중 (Live Load)



○ 충격계수 :  $15 / (40+L) = 0.356 \implies 0.300$

○ 차륜하중 :  $Pr = 72 \text{ kN}$

○ P (활하중) =  $Pr \times (1+0.3) \times \Sigma(\text{종거 : 종단면})$

○ M (모멘트) =  $P \times \Sigma(\text{종거 : 모멘트})$

○ S (전단력) =  $P \times \Sigma(\text{종거 : 전단력})$

| 구 분   | P <sub>1</sub> | P <sub>2</sub> | P <sub>3</sub> | Σ 종거  | 단 면 력      |
|-------|----------------|----------------|----------------|-------|------------|
| a) 종거 | 1.000          | 0.717          | 0.434          | 2.152 | P = 201.38 |
| b) 종거 | 0.538          | -              | -              | 0.538 | M = 108.24 |
| c) 종거 | 1.000          | 0.163          | -              | 1.163 | S = 234.16 |

#### ③ 단 면 력

$M_u = 1.3 M_d + 2.15 M_l = 260.72 \text{ kN-m}$

$S_u = 1.3 S_d + 2.15 S_l = 555.54 \text{ kN}$

### 1.1.2 철근량 산정

|                         |        |                        |      |
|-------------------------|--------|------------------------|------|
| f <sub>ck</sub> (MPa) : | 21     | f <sub>y</sub> (MPa) : | 300  |
| M <sub>u</sub> (kN-m) : | 260.72 | φ (bending) :          | 0.85 |
| S <sub>u</sub> (kN) :   | 555.54 | φ (shear) :            | 0.80 |
| b (mm) :                | 450    | e (mm) :               | 0    |
| d (mm) :                | 1650   | H (mm) :               | 1700 |

#### ㉠ 휨 검토

$$A_{s\_req} = \frac{\phi d f_y - \sqrt{\{(\phi d f_y)^2 - 4AM_u\}}}{2A} = 624 \text{ mm}^2$$

$$\text{여기서, } A = \frac{\phi (f_y)^2}{1.7 f_{ck} b} = 4.762$$

$$1\text{단배근: } D29 \quad 2EA \Rightarrow 1,285 \text{ mm}^2$$

$$A_{s\_use} = 1,285 \text{ mm}^2, \quad p_{\_use} = A_{s\_use} / b d = 0.173\%$$

$$A_{s\_min} = 1.4 b d / f_y = 3,465 \text{ mm}^2, \quad A_{s\_req} \times 4/3 = 832 \text{ mm}^2$$

$$A_{s\_use} (1,285 \text{ mm}^2) > \text{MIN.}(A_{s\_min}, A_{s\_req} \times 4/3) = 832 \text{ mm}^2 \Rightarrow 0.K$$

$$p_{\_min} = 1.4/f_y = 0.467\%, \quad p_{\_max} = 2.529\%$$

$$p_{\_use}=0.173(\%) < p_{\_min}=0.467(\%), \text{ 그러나 } A_{s\_use} > A_{s\_req} \times 4/3 \text{ 이므로} \Rightarrow 0.K$$

$$a = A_{s\_use} f_y / (0.85 f_{ck} b) = 48.0 \text{ mm}$$

$$\phi M_n = 532.719 \text{ kN-m} > M_u = 260.720 \text{ kN-m} \Rightarrow 0.K$$

#### ㉡ 전단 검토

$$L / d = 1.303 < 5 \Rightarrow \text{Deep Beam}$$

$$\phi V_c = 453.67 \text{ kN} \quad \phi V_s = 101.87 \text{ kN}$$

$$V_u = 555.54 \text{ kN} \quad s_{\_max} = 600 \text{ mm}$$

$V_u > \phi V_c$  이므로 전단철근이 부담하여야 하는  $V_s$ 에 대한 철근량 산정

$$A_v = D13 \quad 2EA = 253 \text{ mm}^2, \quad s = 250 \text{ mm}$$

$$A_h = D29 \quad 2EA = 1,285 \text{ mm}^2, \quad s_2 = 300 \text{ mm}$$

$$A_v = 253 > 0.0015 b s = 169 \Rightarrow 0.K$$

$$A_h = 1,285 > 0.0025 b s_2 = 338 \Rightarrow 0.K$$

$$0.192 A_v / s + 0.808 A_h / s_2 = 3.655 > 0.26 \quad (= \phi V_s / \phi f_y d) \Rightarrow 0.K$$

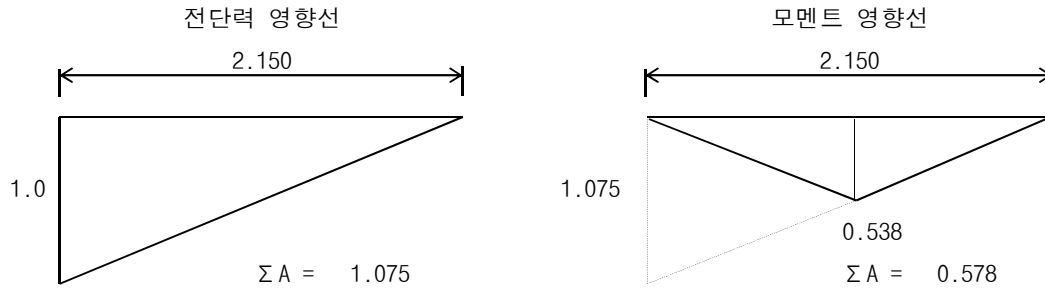
## 2. 가로보 검토(강도설계법) B =1700mm, T = 1000mm (DB-18)

### 2.1 연속부 가로보

#### 2.1.1 작용 단면력

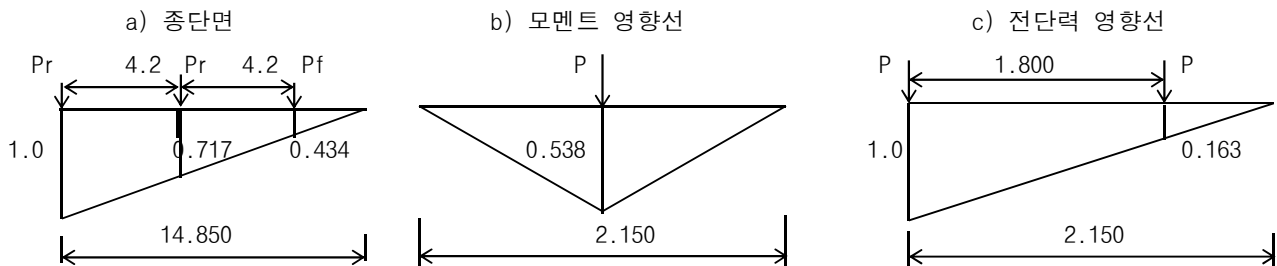
빔간격 = 2.300 m , L = 2.150 m

#### ① 고정하중 (Dead Load)



| 구 분 | 두 개   | 폭     | 단위중량  | 전단력    | 종 거   | 모멘트    |
|-----|-------|-------|-------|--------|-------|--------|
| 포 장 | 0.080 | 5.960 | 2,300 | 11.789 | 0.538 | 6.337  |
| 슬래브 | 0.170 | 5.960 | 2,500 | 27.230 | 0.538 | 14.636 |
| 가로보 | 1.700 | 1.000 | 2,500 | 45.688 | 0.538 | 24.557 |
| 합 계 |       |       |       | 84.706 |       | 45.530 |

#### ② 활 하 중 (Live Load)



○ 충격계수 :  $15 / (40+L) = 0.356 \Rightarrow 0.300$

○ 차륜하중 :  $Pr = 72 \text{ kN}$

○ P (활하중) =  $Pr \times (1+0.3) \times \Sigma(\text{종거 : 종단면})$

○ M (모멘트) =  $P \times \Sigma(\text{종거 : 모멘트})$

○ S (전단력) =  $P \times \Sigma(\text{종거 : 전단력})$

| 구 분   | P <sub>1</sub> | P <sub>2</sub> | P <sub>3</sub> | Σ 종거  | 단 면 력      |
|-------|----------------|----------------|----------------|-------|------------|
| a) 종거 | 1.000          | 0.717          | 0.434          | 2.152 | P = 201.38 |
| b) 종거 | 0.538          | -              | -              | 0.538 | M = 108.24 |
| c) 종거 | 1.000          | 0.163          | -              | 1.163 | S = 234.16 |

#### ③ 단 면 력

$M_u = 1.3 M_d + 2.15 M_l = 291.91 \text{ kN-m}$

$S_u = 1.3 S_d + 2.15 S_l = 613.57 \text{ kN}$

### 2.1.2 철근량 산정

|             |        |               |      |
|-------------|--------|---------------|------|
| fck (MPa) : | 21     | fy (MPa) :    | 300  |
| Mu (kN-m) : | 291.91 | φ (bending) : | 0.85 |
| Su (kN) :   | 613.57 | φ (shear) :   | 0.80 |
| b (mm) :    | 1000   | e (mm) :      | 0    |
| d (mm) :    | 1650   | H (mm) :      | 1700 |

#### ㉠ 휨 검토

$$A_{s\_req} = \frac{\phi d f_y - \sqrt{\{(\phi d f_y)^2 - 4AMu\}}}{2A} = 696 \text{ mm}^2$$

$$\text{여기서, } A = \frac{\phi (f_y)^2}{1.7 f_{ck} b} = 2.143$$

$$1\text{단배근: } D29 \quad 2EA \Rightarrow 1,285 \text{ mm}^2$$

$$A_{s\_use} = 1,285 \text{ mm}^2, \quad p_{\_use} = A_{s\_use} / b d = 0.078\%$$

$$A_{s\_min} = 1.4 b d / f_y = 7,700 \text{ mm}^2, \quad A_{s\_req} \times 4/3 = 928 \text{ mm}^2$$

$$A_{s\_use} (1,285 \text{ mm}^2) > \text{MIN.}(A_{s\_min}, A_{s\_req} \times 4/3) = 928 \text{ mm}^2 \Rightarrow \text{O.K.}$$

$$p_{\_min} = 1.4/f_y = 0.467\%, \quad p_{\_max} = 2.529\%$$

$$p_{\_use}=0.078(\%) < p_{\_min}=0.467(\%), \text{ 그러나 } A_{s\_use} > A_{s\_req} \times 4/3 \text{ 이므로 } \Rightarrow \text{O.K.}$$

$$a = A_{s\_use} f_y / (0.85 f_{ck} b) = 21.6 \text{ mm}$$

$$\phi M_n = 537.042 \text{ kN-m} > M_u = 291.910 \text{ kN-m} \Rightarrow \text{O.K.}$$

#### ㉢ 전단 검토

$$L / d = 1.303 < 5 \Rightarrow \text{Deep Beam}$$

$$\phi V_c = 1008.17 \text{ kN} \quad \phi V_s = 0.00 \text{ kN}$$

$$V_u = 613.57 \text{ kN} \quad s_{\_max} = 600 \text{ mm}$$

$$1/2 \phi V_c < V_u < \phi V_c \text{ 이므로 최소량의 전단철근 보강}$$

$$A_v = D13 \quad 2EA = 253 \text{ mm}^2, \quad s = 150 \text{ mm}$$

$$A_h = D29 \quad 2EA = 1,285 \text{ mm}^2, \quad s_2 = 300 \text{ mm}$$

$$A_v = 253 > 0.0015 b s = 225 \Rightarrow \text{O.K.}$$

$$A_h = 1,285 > 0.0025 b s_2 = 750 \Rightarrow \text{O.K.}$$

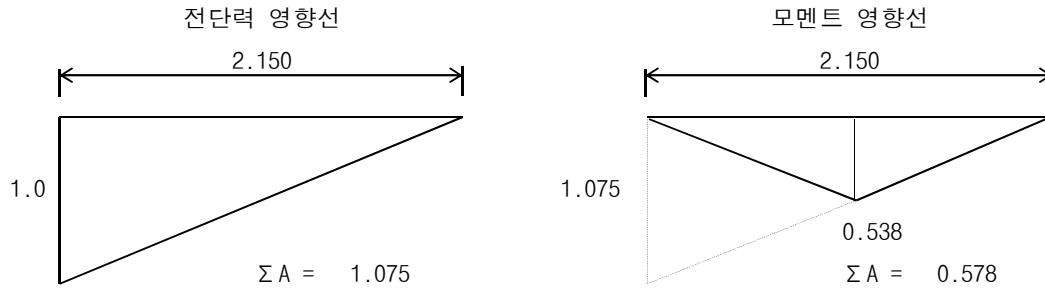
### 3. 가로보 검토(강도설계법) B =1700mm, T = 200mm (DB-18)

#### 3.1 중앙부 가로보

##### 3.1.1 작용 단면력

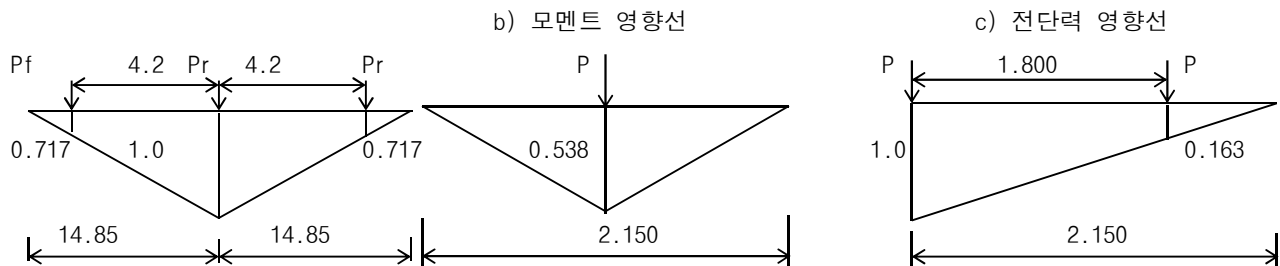
빔간격 = 2.300 m , L = 2.150 m

##### ① 고정하중 (Dead Load)



| 구 분 | 두 개   | 폭     | 단위중량  | 전단력    | 중 거   | 모멘트    |
|-----|-------|-------|-------|--------|-------|--------|
| 포 장 | 0.080 | 5.960 | 2,300 | 11.789 | 0.538 | 6.337  |
| 슬래브 | 0.170 | 5.960 | 2,500 | 27.230 | 0.538 | 14.636 |
| 가로보 | 1.700 | 0.200 | 2,500 | 9.138  | 0.538 | 4.911  |
| 합 계 |       |       |       | 48.156 |       | 25.884 |

##### ② 활 하 중 (Live Load)



○ 충격계수 :  $15 / (40+L) = 0.356 \Rightarrow 0.300$

○ 차륜하중 :  $P_r = 72 \text{ kN}$

○ P (활하중) =  $P_r \times (1+0.3) \times \Sigma(\text{중거 : 종단면})$

○ M (모멘트) =  $P \times \Sigma(\text{중거 : 모멘트})$

○ S (전단력) =  $P \times \Sigma(\text{중거 : 전단력})$

| 구 분   | P <sub>1</sub> | P <sub>2</sub> | P <sub>3</sub> | $\Sigma$ 중거 | 단 면 력      |
|-------|----------------|----------------|----------------|-------------|------------|
| a) 중거 | 0.717          | 1.000          | 0.717          | 2.434       | P = 227.85 |
| b) 중거 | 0.538          | -              | -              | 0.538       | M = 122.47 |
| c) 중거 | 1.000          | 0.163          | -              | 1.163       | S = 264.95 |

##### ③ 단 면 력

$M_u = 1.3 M_d + 2.15 M_l = 296.96 \text{ kN-m}$

$S_u = 1.3 S_d + 2.15 S_l = 632.24 \text{ kN}$

### 3.1.2 철근량 산정

|             |        |               |      |
|-------------|--------|---------------|------|
| fck (MPa) : | 21     | fy (MPa) :    | 300  |
| Mu (kN-m) : | 296.96 | φ (bending) : | 0.85 |
| Su (kN) :   | 632.24 | φ (shear) :   | 0.80 |
| b (mm) :    | 200    | e (mm) :      | 0    |
| d (mm) :    | 1650   | H (mm) :      | 1700 |

#### ㉠ 휨 검토

$$A_{s\_req} = \frac{\phi d f_y - \sqrt{\{(\phi d f_y)^2 - 4AMu\}}}{2A} = 719 \text{ mm}^2$$

$$\text{여기서, } A = \frac{\phi (f_y)^2}{1.7 f_{ck} b} = 10.714$$

$$1\text{단배근: } D29 \quad 2EA \Rightarrow 1,285 \text{ mm}^2$$

$$A_{s\_use} = 1,285 \text{ mm}^2, \quad p_{\_use} = A_{s\_use} / b d = 0.389\%$$

$$A_{s\_min} = 1.4 b d / f_y = 1,540 \text{ mm}^2, \quad A_{s\_req} \times 4/3 = 959 \text{ mm}^2$$

$$A_{s\_use} (1,285 \text{ mm}^2) > \text{MIN.}(A_{s\_min}, A_{s\_req} \times 4/3) = 959 \text{ mm}^2 \Rightarrow 0.K$$

$$p_{\_min} = 1.4/f_y = 0.467\%, \quad p_{\_max} = 2.529\%$$

$$p_{\_use}=0.389(\%) < p_{\_min}=0.467(\%), \text{ 그러나 } A_{s\_use} > A_{s\_req} \times 4/3 \text{ 이므로 } \Rightarrow 0.K$$

$$a = A_{s\_use} f_y / (0.85 f_{ck} b) = 108.0 \text{ mm}$$

$$\phi M_n = 522.893 \text{ kN-m} > M_u = 296.964 \text{ kN-m} \Rightarrow 0.K$$

#### ㉢ 전단 검토

$$L / d = 1.303 < 5 \Rightarrow \text{Deep Beam}$$

$$\phi V_c = 201.63 \text{ kN} \quad \phi V_s = 430.61 \text{ kN}$$

$$V_u = 632.24 \text{ kN} \quad s_{\_max} = 600 \text{ mm}$$

$V_u > \phi V_c$  이므로 전단철근이 부담하여야 하는  $V_s$ 에 대한 철근량 산정

$$A_v = D13 \quad 2EA = 253 \text{ mm}^2, \quad s = 150 \text{ mm}$$

$$A_h = D29 \quad 2EA = 1,285 \text{ mm}^2, \quad s_2 = 300 \text{ mm}$$

$$A_v = 253 > 0.0015 b s = 45 \Rightarrow 0.K$$

$$A_h = 1,285 > 0.0025 b s_2 = 150 \Rightarrow 0.K$$

$$0.192 A_v / s + 0.808 A_h / s_2 = 3.785 > 1.087 \quad (= \phi V_s / \phi f_y d) \Rightarrow 0.K$$